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# Drainage Report

Proposed Development  
Percival Avenue, Berlin CT

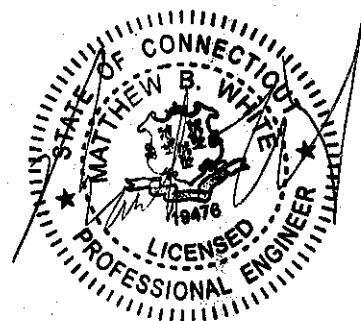
Property of:  
The Town of Berlin

Prepared for:  
The Berlin Housing Authority

Date: July 17, 2014

1-23-15

ajm/s



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## Project Overview

The Housing Authority of the Town of Berlin is proposing a renovation of the former Knights of Columbus facility located on Percival Ave. The site is an  $8.8 \pm \text{Ac}$  parcel located east of Percival Avenue and north of Lawdale Street in Berlin, Connecticut. The site presently has a vacant building and parking lots in the western portion of the property and the eastern portion of the property is wooded and undeveloped. The two sides are divided by a central wetland corridor which flows to a 24" concrete pipe that drains to the north. The property is surrounded by residential neighborhoods to the south and west, a town park to the north and the eastern boundary is bordered by a second wetland corridor that separates the property from a senior housing facility operated by the Housing Authority. A sanitary sewer main (located within a sewer easement) owned and operated by the Kensington Fire District runs through the center of the property, and will remain following the proposed development.

At this time the Housing Authority is planning to remove the existing structures on the site and construct of two main buildings which will contain 50 senior housing units along with the associated drainage, parking facilities, an accessory maintenance building and Gazebo.

## Drainage Area Characteristics

The site was analyzed using the TR-55 method in its existing and developed states to determine peak rates of runoff produced by 2, 10, 25, 50 and 100 yr storm events.

Under existing and developed conditions the site drains to a design point at a 24" concrete pipe inlet located along the northern property boundary in the center of the site. This pipe drains north under the outfield of the baseball field on in the neighboring town park and outlets into a large inland wetland.

## Existing Drainage Areas

The existing drainage area which drains to through the site extends approximately 1,750 ft south west of the design point, nearly to the intersection of Percival Ave. and Sheldon Street, and encompasses 20.76 Ac. The majority of the existing drainage area is located off site and is comprised of residential neighborhoods and wooded areas.

A drainage model was created using Haestad Methods PondPack to determine the peak rate of runoff to the design point in the existing and developed conditions. Drainage area maps which show the existing and developed drainage areas and the design point have been included with this report.

Table 1 shows the input data that was used to determine peak rates of runoff to the design point under existing conditions:



Table 1 – Input data for existing peak rates of runoff

Drainage Area	Curve Number	Area (Ac.)	Time of Concentration (Hr.)
Existing	67	20.76	.54

Table 2 shows the peak rates of runoff that were calculated using PondPack models:

Table 2 – Peak rates of runoff under existing conditions

	Existing Peak Runoff (cfs)				
	2 yr	10 yr	25 yr	50 yr	100 yr
Existing to Design Point	8.34	22.75	28.58	35.68	44.10

### Developed Conditions

#### *Developed to Design Point*

The developed area draining to the design point is divided into two drainage sub areas, Developed and Developed Remaining.

The Developed drainage area includes the majority of the proposed development on the site as well as some of the offsite areas immediately upgradient of the development. Runoff in this area, as well as the roof water from the new buildings, will be collected through a catch basin network that outlets into in a proposed surface detention basin. Abtech DI 1420 N Storm filter inlet protection will be installed in some of the proposed as a water quality measure.

This detention basin will have an internal capacity of .50 Ac-ft ± and will outlet to the Developed Remaining drainage area in the vicinity of the Design Point. An outlet control structure in the basin will reduce developed peak rates of runoff leaving the site at the design point to levels below existing levels.

Developed Remaining will include the remaining area that is left over from the existing drainage area after the Developed subarea has been removed. Developed Remaining is comprised primarily of the residential neighborhood and wooded areas that are located up gradient of the proposed development, these areas bypass the detention basin.

Table 3 shows the input data that was used to determine peak rates of runoff to each of the design points under developed conditions:



Table 3 – Input data for developed peak rates of runoff

Drainage Area	Curve Number	Area (Ac.)	Time of Concentration (Hr.)
Developed	83	3.43	.19
Developed Remaining	66	17.33	0.54

Table 4 shows the peak rates of runoff that were calculated using PondPack models:

Table 4 – Peak rates of runoff under developed conditions

	Developed Peak Runoff (cfs)				
	2 yr	10 yr	25 yr	50 yr	100 yr
Developed to Design Point	7.16	22.09	27.97	35.14	43.13
Max. Water Surface Elevation	129.6	130.5	130.7	131.0	131.4

### Conclusion

Drainage area maps as well as data sheets that were created using the stormwater modeling software PondPack are included in the rear of this report.

The proposed stormwater measures that will be constructed as part of the site development will collect and detain the stormwater runoff produced by the addition of impervious areas to the site. Peak rates of runoff will be reduced to below existing levels as a result of the stormwater control measures that will be built on the site as part of the proposed development.



# Appendix



Project	Berlin Housing Authority	Job Number	135948
Location	Percival Ave, Berlin	Date	6/1/2014
Check One		Existing <input type="checkbox"/> or Developed <input checked="" type="checkbox"/>	Print Date 5/30/2014

Check One

Existing	Sub Areas	Total (ft <sup>2</sup> )	Total (Acres)	% of Total Area	Hydraulic Soil Group	Cover Type	Curve Number	Product CN x Area
	50	298	5,611	640	6,599	0.15	0.73%	A
			50,826		50,826	1.17	5.62%	A
								Lawn (Good)
	741							Woods (Good)
								Impervious
	176,783							
			176,783		176,783	4.06	19.55%	A
								1/3 Acre Residential
	2,178							
	10,210							
	2,306							
	143,344							
			158,038		158,038	3.63	17.47%	B
								Lawn (Good)
	102,683							Woods (Good)
								Impervious
	1,923							
	60,056							
		61,979						
			1,42		6,85%	B		
	11,512							
		11,512						
			11,512		0.00	B		
	14,030							
		14,030						
			14,030		0.32	1.27%	C	
	116							
		468						
			468		0.01	0.00%	C	
	276,180							
		276,180						
			276,180		6,34	30.54%	C	
	44,482							
		44,482						
			44,482		0.00	0.00%	D	
	904,437							
		904,437						
			20,76		1.00			

904,437 20,76 1.00

1,400

21 67

CN (Weighted) =

1,400

21

67

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\*\*\*\*\*MASTER SUMMARY\*\*\*\*\*

Watershed..... Master Network Summary ..... 1.01

\*\*\*\*\*RUNOFF HYDROGRAPHS\*\*\*\*\*

EXISTING..... 2  
SCS Unit Hyd. Summary ..... 2.01

Type.... SCS Unit Hyd. Summary  
Name.... EXISTING Tag: 2  
File... C:\HAEESTAD\PPKW\135948 - BERLIN HOUSING\EXISTING.PPW  
Storm... TypeIII 24hr Tag: 2

Page 2.01  
Event: 2 yr

#### SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm  
Duration = 24.0000 hrs Rain Depth = 3.3000 in  
Rain Dir = C:\HAEESTAD\PPKW\RAINFALL  
Rain File - ID = SCSTYPES.RNF - TypeIII 24hr  
Unit Hyd Type = Default-Curvilinear  
HYG Dir = C:\HAEESTAD\PPKW\135948 - BERLIN HOUSING\  
HYG File - ID = - EXISTING 2  
Tc = .5400 hrs  
Drainage Area = 20.760 acres Runoff CN= 67

=====  
Computational Time Increment = .07200 hrs  
Computed Peak Time = 12.4560 hrs  
Computed Peak Flow = 8.36 cfs

Time Increment for HYG File = .0500 hrs  
Peak Time, Interpolated Output = 12.4500 hrs  
Peak Flow, Interpolated Output = 8.34 cfs  
=====

#### DRAINAGE AREA

-----  
ID:None Selected  
CN = 67  
Area = 20.760 acres  
S = 4.9254 in  
0.25 = .9851 in

#### Cumulative Runoff

-----  
.7401 in  
1.280 ac-ft

HYG Volume... 1.280 ac-ft (area under HYG curve)

#### \*\*\*\*\* UNIT HYDROGRAPH PARAMETERS \*\*\*\*\*

Time Concentration, Tc = .54000 hrs (ID: None Selected)  
Computational Incr, Tm = .07200 hrs = 0.20000 Tp

Unit Hyd. Shape Factor = 483.432 (37.46% under rising limb)  
K = 483.43/645.333, K = .7491 (also, K = 2/(1+(Tr/Tp)))  
Receding/Rising, Tr/Tp = 1.6698 (solved from K = .7491)

Unit peak, qp = 43.56 cfs  
Unit peak time Tp = .36000 hrs  
Unit receding limb, Tr = 1.44000 hrs  
Total unit time, Tb = 1.80000 hrs

Project	Berlin Housing Authority	Job Number	135948
Location	Percival Ave, Berlin	Date	12/24/2013

Print Date  
5/30/2014

Check One

Existing       Developed

Developed to Basin	Sub Areas	Total (ft <sup>2</sup> )	Total (Acres)	% of Total Area	Hydraulic Soil Group	Cover Type	Curve Number	Product CN x Area
		0	0.00%	A	Lawn (Good)	39	0.0	
		0	0.00%	A	Woods (Good)	30	0.0	
		0	0.00%	A	Impervious	98	0.0	
		0	0.00%	A	1/3 Acre Residential	57	0.0	
28,750	23,522	52,272	1.20	34.96%	B	Lawn (Good)	61	73.2
8,712	8,712	0.20	5.83%	B	Woods (Good)	55	11.0	
10,890	77,666	88,556	2.03	59.22%	B	Impervious	98	199.2
		0	0.00%	B	1/3 Acre Residential	72	0.0	
		0	0.00%	C	Lawn (Good)	74	0.0	
		0	0.00%	C	Woods (Good)	70	0.0	
		0	0.00%	C	Impervious	98	0.0	
		0	0.00%	C	1/3 Acre Residential	81	0.0	
		0	0.00%	D	Lawn (Good)	80	0.0	
		0	0.00%	D	Woods (Good)	77	0.0	
		0	0.00%	D	Impervious	98	0.0	
		0	0.00%	D	1/3 Acre Residential	86	0.0	
149,540		3.43	1.00					283

CN (Weighted) =  $\frac{283}{3} = \boxed{83}$

Developed Remaining	Sub Areas	Total (ft <sup>2</sup> )	Total (Acres)	% of Total Area	Hydraulic Soil Group	Cover Type	Curve Number	Product CN x Area	
		51,401	0	0.00%	A	Lawn (Good)	39	0.0	
		51,401	51,401	1.18	A	Woods (Good)	30	35.4	
		0	0	0.00%	A	Impervious	98	0.0	
		174,240	174,240	4.00	23.08%	A	1/3 Acre Residential	57	228.0
-28,750	49,223	52,272	72,745	1.67	9.64%	B	Lawn (Good)	61	101.9
-8,712	54,450	55,321	101,059	2.32	13.39%	B	Woods (Good)	55	127.6
-10,890	10,890	0	0	0.00%	B	Impervious	98	0.0	
		0	0	0.00%	B	1/3 Acre Residential	72	0.0	
	4,792	4,792	0.14	0.63%	C	Lawn (Good)	74	8.1	
		20,909	20,909	0.48	2.77%	C	Woods (Good)	70	33.6
		0	0	0.00%	C	Impervious	98	0.0	
280,526	280,526	6.44	37.16%	C	1/3 Acre Residential	81	521.6		
		0	0	0.00%	D	Lawn (Good)	80	0.0	
	49,223	49,223	1.13	6.52%	D	Woods (Good)	77	87.0	
		0	0	0.00%	D	Impervious	98	0.0	
		0	0	0.00%	D	1/3 Acre Residential	86	0.0	
		754,894	17.33	1.00				1,143	

$$CN \text{ (Weighted)} = \frac{1,143}{17} \boxed{66}$$

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#### \*\*\*\*\*MASTER SUMMARY\*\*\*\*\*

Watershed..... Master Network Summary ..... 1.01

#### \*\*\*\*\*RUNOFF HYDROGRAPHS\*\*\*\*\*

##### DEVEL. REMAINING 2

SCS Unit Hyd. Summary ..... 2.01

##### DEVELOPED..... 2

SCS Unit Hyd. Summary ..... 2.02

#### \*\*\*\*\*POND VOLUMES\*\*\*\*\*

BASIN 1..... Vol: Elev-Area ..... 3.01

#### \*\*\*\*\*OUTLET STRUCTURES\*\*\*\*\*

PR 10..... Outlet Input Data ..... 4.01

DEVEL. REMAINING AREA	25	3.102	12.4000	22.84
DEVEL. REMAINING AREA	50	3.848	12.4000	28.68

S/N: 521301306A81  
PondRack Ver. 7.5 (767)

Angus McDonald / Gary Sharpe  
Compute Time: 13:45:40 Date: 05/28/2014

Type... SCS Unit Hyd. Summary  
Name... DEVEL. REMAINING Tag: 2  
File... C:\HAESTAD\PPKW\135948 - BERLIN HOUSING\CURRENT PLANS\DEVELOPED WITH POND -  
OPTION C - REVISED BASIN.PPW  
Storm... TypeIII 24hr Tag: 2

Page 2.01  
Event: 2 yr

#### SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm  
Duration = 24.0000 hrs Rain Depth = 3.3000 in  
Rain Dir = C:\HAESTAD\PPKW\RAINFALL\  
Rain File ID = SCSTYPES.RNF TypeIII 24hr  
Unit Hyd Type = Default Curvilinear  
HYG Dir = C:\HAESTAD\PPKW\135948 - BERLIN HOUSING\CURRENT PLANS\  
HYG File, - ID = - DEVEL. REMAINING 2  
Tc = .5400 hrs  
Drainage Area = 17.330 acres Runoff CN= 66

=====

Computational Time Increment = .07200 hrs  
Computed Peak Time = 12.4560 hrs  
Computed Peak Flow = 6.39 cfs

Time Increment for HYG File = .0500 hrs  
Peak Time, Interpolated Output = 12.4500 hrs  
Peak Flow, Interpolated Output = 6.37 cfs

=====

#### DRAINAGE AREA

-----  
ID:None Selected  
CN = 66  
Area = 17.330 acres  
S = 5.1515 in  
0.2S = 1.0303 in

#### Cumulative Runoff

.6942 in  
1.002 ac-ft

HYG Volume... 1.002 ac-ft (area under HYG curve)

#### \*\*\*\*\* UNIT HYDROGRAPH PARAMETERS \*\*\*\*\*

Time Concentration, Tc = .54000 hrs (ID: None Selected)  
Computational Incr, Tm = .07200 hrs = 0.20000 Tp

Unit Hyd. Shape Factor = 483.432 (37.46% under rising limb)  
K = 483.43/645.333, K = .7491 (also, K = 2/(1+(Tr/Tp))  
Receding/Rising, Tr/Tp = 1.6698 (solved from K = .7491)

Unit peak, qp = 36.36 cfs  
Unit peak time Tp = .36000 hrs  
Unit receding limb, Tr = 1.44000 hrs  
Total unit time, Tb = 1.80000 hrs

Type.... Vol: Elev-Area  
Name.... BASIN 1

Page 3.01

File.... C:\HAESTAD\PPKW\135948 - BERLIN HOUSING\CURRENT PLANS\DEVELOPED WITH POND -  
OPTION C - REVISED BASIN.PPW

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqrt(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
126.50	----	.0200	.0000	.000	.000
127.00	----	.0400	.0883	.015	.015
128.00	----	.0600	.1490	.050	.064
130.00	----	.1300	.2783	.186	.250
131.00	----	.1700	.4487	.150	.399
132.00	----	.2000	.5544	.185	.584

#### POND VOLUME EQUATIONS

\* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2}-\text{EL1}) * (\text{Area1} + \text{Area2} + \sqrt{\text{Area1} * \text{Area2}})$$

where: EL1, EL2 = Lower and upper elevations of the increment

Area1, Area2 = Areas computed for EL1, EL2, respectively

Volume = Incremental volume between EL1 and EL2

Type.... Outlet Input Data  
Name.... PR 10

Page 4.02

File.... C:\HAESTAD\PPKW\135948 - BERLIN HOUSING\CURRENT PLANS\DEVELOPED WITH POND -  
OPTION C - REVISED BASIN.PPW

#### OUTLET STRUCTURE INPUT DATA

Structure ID =  
Structure Type = Orifice-Circular  
-----  
# of Openings = 1  
Invert Elev. = 126.50 ft  
Diameter = .3300 ft  
Orifice Coeff. = .600

Structure ID =  
Structure Type = Orifice-Circular  
-----  
# of Openings = 1  
Invert Elev. = 130.25 ft  
Diameter = .7500 ft  
Orifice Coeff. = .600

Structure ID =  
Structure Type = Orifice-Circular  
-----  
# of Openings = 1  
Invert Elev. = 129.25 ft  
Diameter = 1.0000 ft  
Orifice Coeff. = .600

Structure ID = TW  
Structure Type = TW SETUP, DS Channel  
-----

#### FREE OUTFALL CONDITIONS SPECIFIED

##### CONVERGENCE TOLERANCES...

Maximum Iterations= 30  
Min. TW tolerance = .01 ft  
Max. TW tolerance = .01 ft  
Min. HW tolerance = .01 ft  
Max. HW tolerance = .01 ft  
Min. Q tolerance = .10 cfs  
Max. Q tolerance = .10 cfs

C numbers	
Woods	0.2
Lawns	0.3
Impervious	0.9

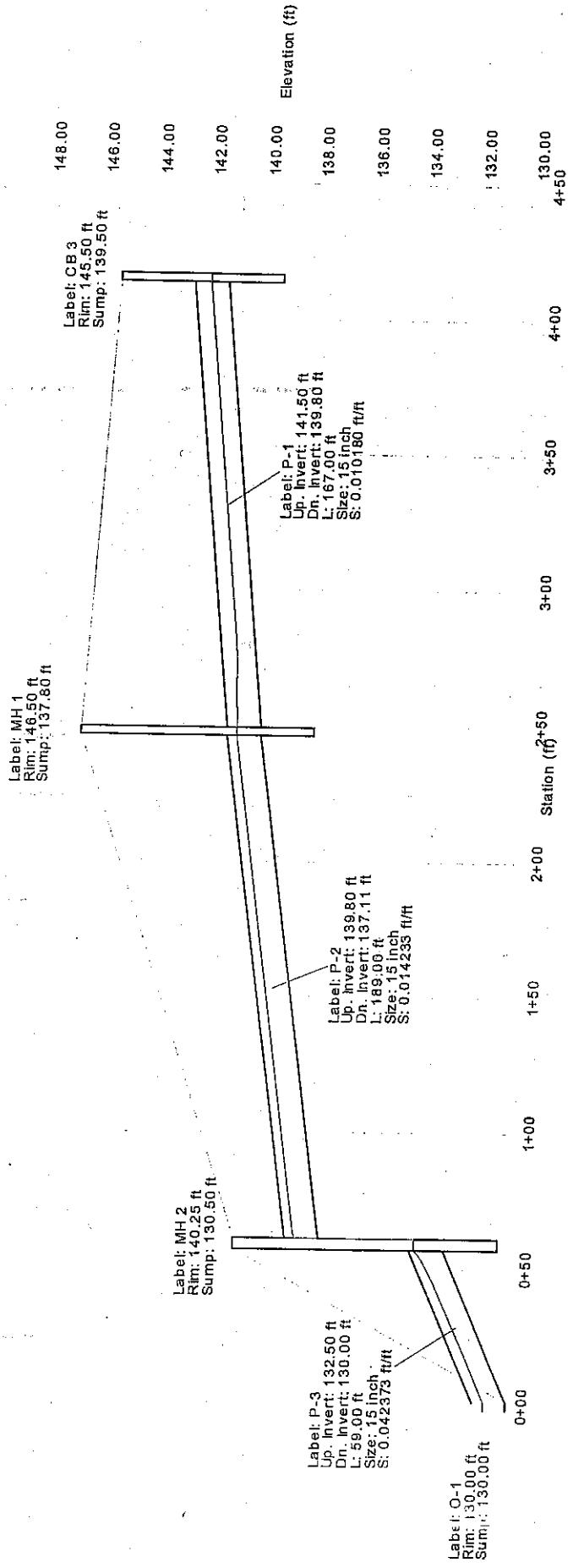
## DRAINAGE AREA

Date: 6/2/2014

Catch Basin Area	Acres			Total Area
	Wooded Area	Lawn Area	Impervious Area	
1	0.00	0.03	0.35	0.38
2	0.00	0.09	0.20	0.29
3	0.20	0.77	0.25	1.22
4	1.05	0.47	0.00	1.52
5	0.00	0.08	0.17	0.25
6	0.00	0.01	0.18	0.19
7	0.00	0.03	0.17	0.20



**Profile**  
**Scenario: 25 Year Storm**



Title: Berlin Housing  
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Project Engineer: McDonald/Sharpe & Associates  
StormCAD v4.1 [428]  
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Scenario: Base

CB 4



P-3

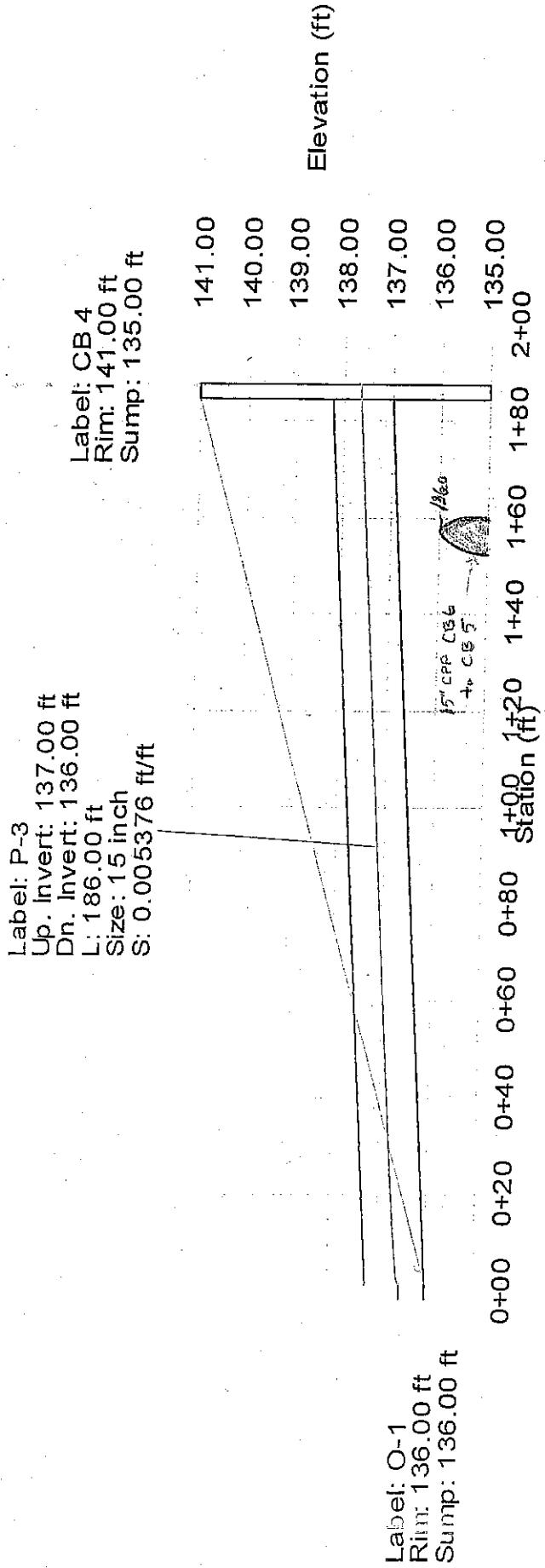
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Title: Berlin Housing  
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Project Engineer: McDaniel Sharpe & Associates  
StormCAD v4.1 [428]  
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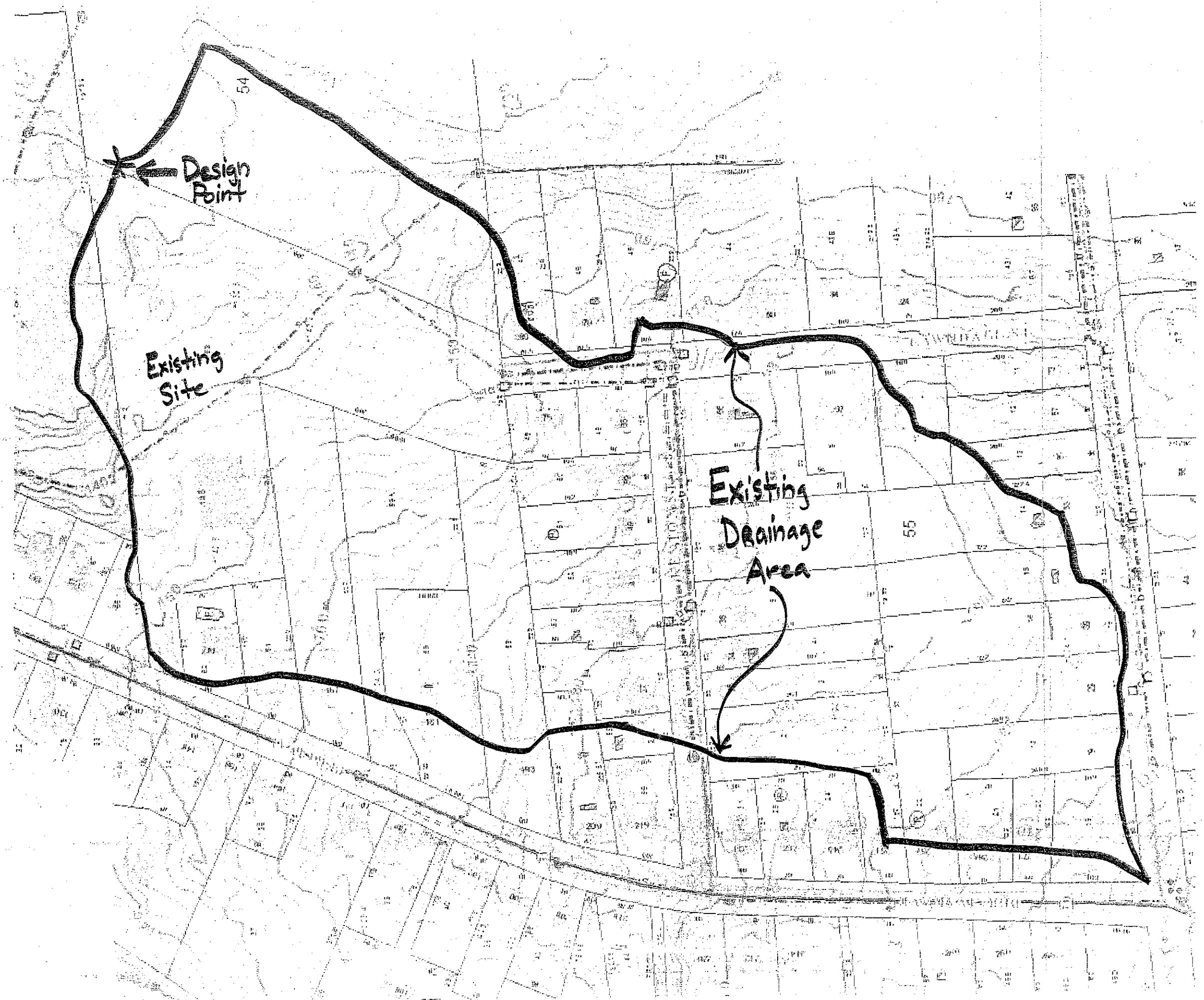
**Profile**  
**Scenario: Base**



Scenario: Base

## STORM DRAINAGE RESULTS

Up Node	OUT	Label	Up Inlet Area (acres)	Up Inlet Rat. Coef.	System CA (acres)	System Q (cfs)	Full Q (cfs)	System Additional Flow (cfs)	Avg. v (ft/s)	Size (ft)	L (ft)	S (ft/ft)	Up Invent (ft)	Dn. Invent (ft)	Up Gr Elev. (ft)	Dn. Gr. Elev. (ft)	HGL In (ft)	HGL Out (ft)
CB6	CB 5	P-4	0.19	0.87	0.17	1.12	4.67	0.00	3.12	15 inch	69.00	0.010000	135.03	134.34	141.20	140.50	135.45	134.76
CB4	CB 7	P-5	0.25	0.71	0.34	2.28	4.67	0.00	3.77	15 inch	135.00	0.010000	131.85	130.50	140.50	139.00	132.47	131.12
CB1	O-1	P-3	0.20	0.81	0.50	3.29	3.89	0.00	4.01	15 inch	36.00	0.006944	130.25	130.00	139.00	130.00	131.12	130.73



Existing  
Drainage  
Area Map

